

Distribution of Strands

Try to use 3 layers of 9 strands in each one with $s = 50 \text{ mm} \rightarrow$ check

$$e_c = 504 \text{ mm} = \text{the assumed value} \therefore OK$$

$$c_b = 70 - 12 - 12.7/2 = 51.65 \text{ mm} > 50 \text{ mm} \therefore OK$$

$$c_s = (700 - 9 \times 12.7 - 8 \times 50)/2 = 92.85 \text{ mm} > 50 \text{ mm} \therefore OK$$

Check of Concrete Stresses

At-Release Stage

$$f_{ti} = 0.63\sqrt{f'_{ci}} = 0.63 \times \sqrt{30} = 3.45 \text{ MPa}$$

$$f_{ci} = 0.6f'_{ci} = 0.6 \times 30 = 18 \text{ MPa}$$

At midspan P_i and M_g .

$$P_i = N_p P_{i,p} = 27 \times 137.68 = 3717.36 \text{ kN}$$

$$M_g = w_g L^2 / 8 = 12.6 \times 24^2 / 8 = 907.2 \text{ kN.m}$$

$$f_{i,top} = -\frac{P_i}{A_g} + \frac{P_i \cdot e}{S_{tg}} - \frac{M_g}{S_{tg}} = -\frac{3717.36 \times 10^3}{525 \times 10^3} + \frac{3717.36 \times 10^3 \times 504}{144.8 \times 10^6} - \frac{907.2 \times 10^6}{144.8 \times 10^6}$$

$$= -7.08 + 12.94 - 6.27 = -0.41 \text{ MPa} < 18 \text{ MPa} \therefore OK$$

$$f_{i,bot} = -\frac{P_i}{A_g} - \frac{P_i \cdot e}{S_{bg}} + \frac{M_g}{S_{bg}} = -\frac{3717.36 \times 10^3}{525 \times 10^3} - \frac{3717.36 \times 10^3 \times 504}{180.07 \times 10^6} + \frac{907.2 \times 10^6}{180.07 \times 10^6}$$

$$= -7.08 - 10.4 + 5.04 = -12.44 \text{ MPa} < 18 \text{ MPa} \therefore OK$$

At ends P_i load only.

$$f_{i,top} = -\frac{P_i}{A_g} + \frac{P_i \cdot e}{S_{tg}} = -7.08 + 12.94 = 5.86 \text{ MPa} > 3.45 \text{ MPa} \therefore NOK$$

\rightarrow debond some strands (N_{dp})

$$N_{dp} = \frac{(f_{i,top} - f_{ti})}{f_{i,top}} \cdot N_p = \frac{(5.86 - 3.45)}{5.86} \times 27 = 11.1 \text{ say } 12 \text{ strands}$$

$$\rightarrow P_{i,eff.} = \frac{(N_p - N_{dp})}{N_p} \cdot P_i = \frac{15}{27} \times 3717.36 = 2065.2 \text{ kN}$$

$$f_{i,top} = -\frac{P_{i,eff.}}{A_g} + \frac{P_{i,eff.} \cdot e}{S_{tg}} = -\frac{2065.2 \times 10^3}{525 \times 10^3} + \frac{2065.2 \times 10^3 \times 504}{144.8 \times 10^6}$$

$$= -3.93 + 7.19 = 3.26 \text{ MPa} < 3.45 \text{ MPa} \therefore OK$$

$$f_{i,bot} = -\frac{P_{i,eff.}}{A_g} - \frac{P_{i,eff.} \cdot e}{S_{bg}} = -\frac{2065.2 \times 10^3}{525 \times 10^3} - \frac{2065.2 \times 10^3 \times 504}{180.07 \times 10^6}$$

$$= -3.93 - 5.78 = -9.71 \text{ MPa} < 18 \text{ MPa} \therefore OK$$

Check if tensile reinforcement is needed at top of beam

$$f_{ti} = 0.25\sqrt{f'_{ci}} = 0.25 \times \sqrt{30} = 1.37 \text{ MPa (without bonded reinforcement)}$$

$$f_{i,top} = 3.26 \text{ MPa} > 1.37 \text{ MPa} \therefore \text{additional bonded reinforcement is needed}$$

from Trigonometric \rightarrow depth of tensile zone = 362 mm

$$T = 200 \times 12 \times \frac{0.11}{2} + \frac{200 + 500}{2} \times 150 \times \frac{0.11 + 1.46}{2} + 500 \times 200 \times \frac{1.46 + 3.26}{2}$$

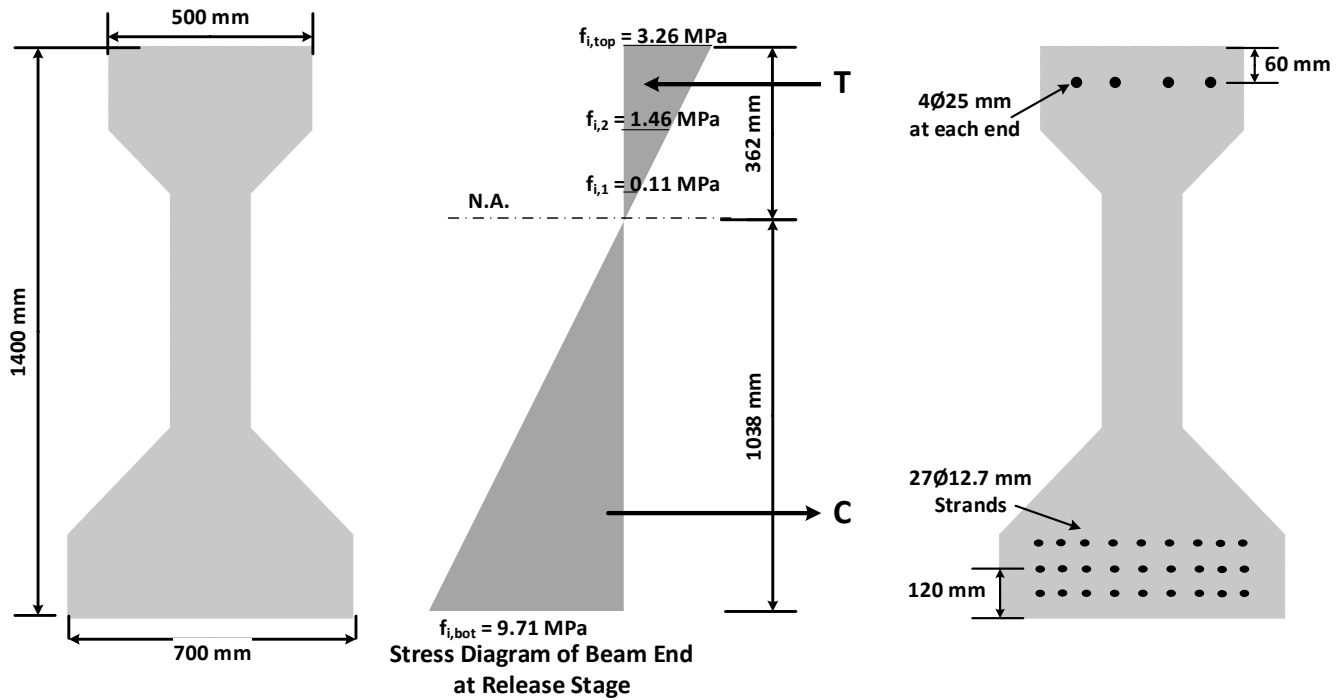
$$= 132 + 41212.5 + 236000 = 277344.5 \text{ N}$$

$$f_y = 420 \text{ MPa} \rightarrow f_s = 170 \text{ MPa}$$

$$A_s = T/f_s = 277344.5/170 = 1631 \text{ mm}^2$$

$$\phi_b = 25 \text{ mm} \rightarrow A_b = 490.87 \text{ mm}^2$$

$$N_b = A_s/A_b = 1631/490.87 = 3.32 \text{ say } 4 \text{ bars}$$



In-Service Stage

$$f_c = 0.45f'_c = 0.45 \times 35 = 15.75 \text{ MPa}$$

$$f_t = 0.50\sqrt{f'_c} = 0.5 \times \sqrt{35} = 2.96 \text{ MPa}$$

At midspan P_e , $M_{D,nc}$, $M_{D,c}$, M_{DW} and $M_{(LL+IM)}$.

$$P_e = R \cdot P_i = 0.8 \times 3717.36 = 2973.89 \text{ kN}$$

$$f_{top} = -\frac{P_e}{A_g} + \frac{P_e \cdot e}{S_{tg}} - \frac{M_{DC1}}{S_{tg}} - \frac{M_{DC2} + M_{DW} + 0.8M_{(LL+IM)}}{S_{tcg}}$$

$$= -\frac{2973.89}{525} + \frac{2973.89 \times 0.504}{144.8} - \frac{1920.24}{144.8} - \frac{180 + 115.2 + 0.8 \times 1955.35}{880.11}$$

$$= -5.66 + 10.35 - 13.26 - 2.11 = -10.68 \text{ MPa} < 15.75 \text{ MPa} \therefore \text{OK}$$

$$f_{bot} = -\frac{P_e}{A_g} - \frac{P_e \cdot e}{S_{bg}} + \frac{M_{DC1}}{S_{bg}} + \frac{M_{DC2} + M_{DW} + 0.8M_{(LL+IM)}}{S_{bcg}}$$

$$= -\frac{2973.89}{525} - \frac{2973.89 \times 0.504}{180.07} + \frac{1920.24}{180.07} + \frac{180 + 115.2 + 0.8 \times 1955.35}{303.97}$$

$$= -5.66 - 8.37 + 10.66 + 6.12 = 2.75 \text{ MPa} < 2.96 \text{ MPa} \therefore \text{OK}$$

At ends P_e load only.

$$\because P_e < P_i \rightarrow P_{e,eff.} < P_{i,eff.} \therefore \text{no need to check}$$