

## 2. Construction Materials Properties

### 2.1. Normal-Weight and Lightweight Concrete

#### 2.1.1. Compressive Strength

The average compressive strength of the cylindrical samples ( $f'_c$ ) at age of 28-day for concrete, must be within:

- $f'_c \geq 2.4$  ksi [structural applications]  
   $\geq 4.0$  ksi [prestressed concrete and decks]  
   $\leq 10.0$  ksi [beyond, tests and allowance are essential]

#### 2.1.2. Cement and Water Content

The sum of Portland cement and other cementitious materials ( $CM$ ) as well as the water-cement ratio ( $W/CM$ ) shall be specified as following:

- $CM \leq 29.6$  lb/ft<sup>3</sup> [ordinary concrete]  
   $\leq 37$  lb/ft<sup>3</sup> [high performance concrete (HPC)]
- $W/CM \leq 0.45$

#### 2.1.3. Coefficient of Thermal Expansion

For more precise data, the coefficient of thermal expansion ( $\alpha$ ) should be determined by laboratory tests. Other else, it may be taken as:

- $\alpha = 10.8 \times 10^{-6}/^\circ\text{C}$  [normal-weight concrete]  
   $= 9.0 \times 10^{-6}/^\circ\text{C}$  [lightweight concrete]

#### 2.1.4. Modulus of Elasticity

In the absence of measured data, the modulus of elasticity ( $E_c$ ) for normal-weight concrete with design compressive strengths ( $f'_c$ ) up to 15.0 ksi and lightweight concrete with up to 10.0 ksi, with unit weight ( $\gamma_c$ ) between 0.090 and 0.155 kip/ft<sup>3</sup>, may be taken as:

- $E_c = 12000K_1 \cdot \gamma_c^{2.0} \cdot f_c'^{0.33}$  [normal-weight and lightweight concrete]  
   $= 2500f_c'^{0.33}$  [normal-weight concrete with  $\gamma_c = 0.145$  kip/ft<sup>3</sup>]

For normal-weight concrete with ( $\gamma_c$ ) = 0.145 kip/ft<sup>3</sup> ( $f'_c$ ) up to 10.0 ksi, ( $E_c$ ) can be taken as:

- $E_c = 33000K_1 \cdot \gamma_c^{1.5} \cdot \sqrt{f'_c}$   
   $= 1820\sqrt{f'_c}$

where:

$K_1$ : correction factor for aggregate source; taken as 1.0 in absent of physical test.

$\gamma_c$ : unit weight of concrete (kip/ft<sup>3</sup>).

$f'_c$ : compressive strength of concrete (ksi).

### 2.1.5. Poisson's Ratio

Unless determined by physical tests, Poisson's ratio ( $\nu$ ) may be assumed as 0.2 for normal-weight concrete with ( $f'_c$ ) up to 15.0 ksi and lightweight concrete with up to 10.0 ksi. While for components expected to be subject to cracking, the effect of ( $\nu$ ) may be neglected.

### 2.1.6. Modulus of Rupture

Unless determined by physical tests, the modulus of rupture ( $f_r$ ) for normal-weight concrete with specified ( $f'_c$ ) up to 15.0 ksi and lightweight concrete with up to 10.0 ksi, may be taken as:

- $f_r = 0.24\lambda \cdot \sqrt{f'_c}$

where:

$\lambda$ : concrete density modification factor; taken as 1.0 for normal-weight concrete.

### 2.1.7. Tensile Strength

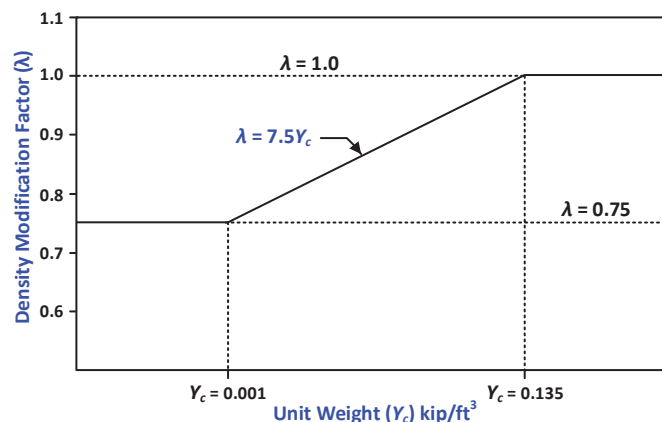
For normal-weight concrete with design ( $f'_c$ ) up to 10.0 ksi, the direct tensile strength ( $f_t$ ) may be estimated as:

- $f_t = 0.23\sqrt{f'_c}$

### 2.1.8. Concrete Density Modification Factor

The concrete density modification factor ( $\lambda$ ) shall be determined with respect to the concrete splitting tensile strength ( $f_{ct}$ ) as:

- $\lambda \geq 0.75$  [lower limit]
- $= 4.7f_{ct}/\sqrt{f'_c} \leq 1.0$  [ $f_{ct}$  is specified]
- $= 7.5\gamma_c \leq 1.0$  [ $f_{ct}$  is not specified]
- $= 1.0$  [normal-weight concrete]



**Figure 2.1: Illustration of Concrete Density Modification Factor as a Function of Unit Weight [AASHTO LRFD Figure C5.5.4.2-1]**

## 2.2. Reinforcing Steel

### 2.2.1. Yield Strength

The reinforcing steel must be deformed and with yield strength ( $f_y$ ):

- $75.0 \leq f_y \leq 100.0$  ksi [design purposes]

Plain steel bars or plain wire may be used for spirals, hoops and wire fabric. Bars with yield strengths ( $f_y$ ) less than 60.0 ksi shall be used only with the approval of the owner.

### 2.2.2. Modulus of Elasticity

The modulus of elasticity ( $E_s$ ) for reinforcing steel shall be assumed as:

- $E_s = 29000$  ksi [ $f_y \leq 100.0$  ksi]

## 2.3. Prestressing Steel

The prestressing steel properties must follow one of the:

- Uncoated, stress-relieved or low-relaxation, seven-wire strand.
- Uncoated plain or deformed, high-strength bars.

### 2.3.1. Yield Strength

The yield strength of the prestressing steel ( $f_{py}$ ) as related to its tensile strength ( $f_{pu}$ ) is specified in Table below:

**Table 2.1: Properties of Prestressing Strand and Bar [AASHTO LRFD Table 5.4.4.1-1]**

Material	Grade or Type	Diameter ( $\phi$ ) in.	Tensile Strength ( $f_{pu}$ ) ksi	Yield Strength ( $f_{py}$ ) ksi
Strand	Grade 270 ksi	0.375 - 0.6	270	$0.90f_{pu}$
Bar	Type 1, Plain	0.75 - 1.375	150	$0.85f_{pu}$
	Type 2, Deformed	0.625 - 2.5	150	$0.80f_{pu}$

### 2.3.2. Modulus of Elasticity

If more precise data are not available, the modulus of elasticity for prestressing ( $E_{ps}$ ) steels, may be taken as:

- $E_{ps} = 28500$  ksi [strands]  
 = 30000 ksi [bars]