CHAPTER 2

- Pore spaces in porous media voids, pores, interstices
- Characterized by size, shape, distribution, and irregularity.

Original interstices – created by geologic process; Alluvium, clay

 Secondary interstices – developed after rock formation; limestone, fractured rock

- Size of interstices
 - Capillary water filled by capillary forces
 - Super capillary capillary forces insignificant
 - Sub capillary water held by adhesive forces

• Specific Surface = $\frac{Surface Area of Grains}{BulkVolume}$, (L⁻¹)

- S_c value more for clay than sand and gravel
- more sp. surface more potential of contaminant removal

Specific Surface (S_s)

Silicate powder Loose sand Soils Sandstone

Limestone

 $\begin{array}{r} -1 \\ \text{Cm}^{-1} \\ 6.8 \times 10^{\text{Cm}} \\ 8.9 \times 10^{3} \\ 1.5 \times 10^{2} - 2.2 \times 10^{2} \\ 2 \times 10^{3} - 4 \times 10^{3} \\ 1.5 \times 10^{6} - 10 \times 10^{5} \\ 0.15 \times 10^{4} - 1.3 \times 10^{4} \end{array}$

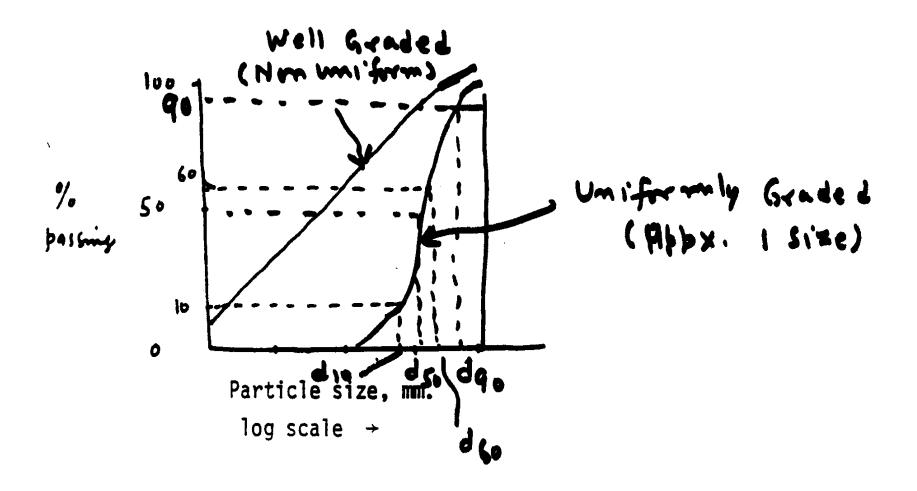
• U.S. Dept Agriculture - Soil Analysis and classification

	<u>Size, mm</u>			
Clay	<	0.002		
Silt		0.002	- 0.05	
Very fine silt		0.05	- 0.1	Fine
sand		0.10	- 0.25	
Medium sand		0.25	- 0.50	
Coarse sand		0.50	- 1.00	
Very coarse sand	1.0	- 2.0		
Gravel	>	2.0		

- SCS classification of gravel
 - Fine gravel 0.6 -- 1.0 cm
 - **Coarse gravel**
 - Small cobbles
 - Large cobbles
 - Boulders

- 1.0 -- 7.6 cm
- 7.6 -- 15.2 cm
- 15.2 -- 30.5 cm
 - > 30.5 cm

 Grain size Distribution (Sieve analysis): Well graded material Uniform graded material



• Effective size (d₁₀) - maximum diameter of the smallest 10% passing by weight.

• Hazen's Uniformity coefficient =



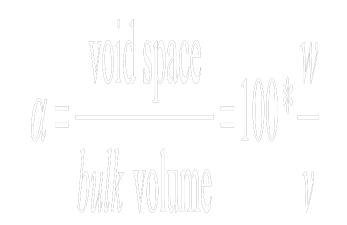
U.C. from 1.0 > 1.0 to

well graded material

uniform material

- When U.C. = 1; d₆₀ = d₁₀ material is one of size
- For well graded material, 5 < U.C. < 10 or more

Porosity - measure of volume of interstices. - express as % of void space to total volume of the mass.



w - volume of water to saturate the pore space

v - total volume of rock or soil (volume of pores & grains)

 α - f (shape, arrangement of grains, size distributions, degree of cementation and compaction)

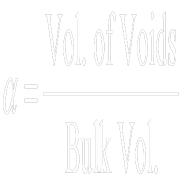
- Unconsolidated formation sand, gravel, silt, clay
- Consolidated formation sandstone, limestone, and igneous rocks

• Porosity changes with fracturing and solution

• Porosity range for sedimentary materials:

Material		α, %
	Soils	50-60
	(unconsolidated material)	
	Clays	45-55
	Silts	40-50
	Sands	30-40
	Gravel & Sand	20-35
	Sandstone	10-20
	Shale & Limestone	1-10

Saturated Zone



Specific Retention - % vol. Of water retained after saturation and drainage



Specific yield - % vol. Of water drained after saturation.

S_y = f(grain size, shape and distribution of pores, compaction of aquifer)

- Specific yield is a fraction of porosity of an aquifer
- Uniform sand, $S_v = 0.30$
- Most alluvial aquifers, S_v = 0.10 0.20

- Approximate values of S_v
 - (from Sacromento Valley, Ca.)

Sy,%, α

- Gravel 25
- Sand & gravel 20 30%
- Fine sand, sandstone 10
- Clay & gravel
 5
- Clay, silt, & fine-grained deposits 3 55%

EXAMPLE 2.2.1

An undisturbed sample of a medium sand weighs 484.68 g. The core of the undisturbed sample is 6 cm in diameter and 10.61 cm high. The sample is oven-dried for 24 hr at 110°C to remove the water content. At the end of the 24 hr, the core sample weighs 447.32 g. Determine the bulk density, void ratio, water content, porosity, and saturation percentage of the sample.

The dry weight of the sample is $W_d = 447.32$ g and the total weight is $W_T = 484.68$ g. The total volume of the undisturbed sample is

$$V_t = \pi r^2 h = \pi (3 \text{ cm})^2 (10.61 \text{ cm}) = 300 \text{ cm}^3$$

The bulk density is defined as the density of solids and voids together, after drying. Thus,

$$\rho_d = \frac{W_d}{V_t} = \frac{447.32 \text{ g}}{300 \text{ cm}^3} = 1.491 \text{ g/cm}^3$$

Assuming quartz is the predominant mineral in the sample, then $\rho_m = 2.65 \text{ g/cm}^3$ Thus, the volume V_s of the solid phase of the sample is

$$V_s = \frac{W_d}{\rho_m} = \frac{447.32 \text{ g}}{2.65 \text{ g/cm}^3} = 168.8 \text{ cm}^3$$

Thus, the total volume of voids in the sample is

$$V_{\nu} = V_{r} - V_{s} = 300 \text{ cm}^{3} - 168.8 \text{ cm}^{3} = 131.2 \text{ cm}^{3}$$

SOLUTION:

With this information, we can calculate the void ratio e of the sample is

$$e = \frac{V_v}{V_s} = \frac{131.2 \text{ cm}^3}{168.8 \text{ cm}^3} = 0.777$$

The volumetric water content of a sample is the volume of the water divided by the volume of the sample

$$\theta_v = \frac{V_{water}}{V_t} = \frac{(W_T - W_d)/\rho_{water}}{V_t} = \frac{484.68 \text{ g} - 447.32 \text{ g}}{300 \text{ cm}^3} / \lg/\text{ cm}^3 = 0.1245 \text{ g/cm}^3 = 0.125$$

where W_w is the total weight of the undisturbed sample before drying. The gravimetric water content of the sample is

$$\theta_w = \frac{W_T - W_d}{W_d} \times 100 = \frac{484.68 \text{ g} - 447.32 \text{ g}}{447.32 \text{ g}} \times 100 = 8.35\%$$

The porosity of the sample is

$$\alpha = \frac{V_{\rm t} - V_{\rm s}}{V_{\rm t}} \times 100 = \frac{300 \,{\rm cm}^3 - 168.8 \,{\rm cm}^3}{300 \,{\rm cm}^3} \times 100 = 43.73\%$$

Finally, the saturation percentage of a sample is defined as the percentage of the pore space that is filled by water,

$$\frac{\theta_{\gamma}}{\alpha} \times 100 = \frac{(0.1245)}{(0.4373)} \times 100 = 28.47\%$$

EXAMPLE 2.2.2

The void ratio of an unconsolidated clay sample is 1.19. Determine the porosity of the sample.

SOLUTION

Using the definition of the void ratio of an undisturbed sample, $e = \frac{V_v}{V_s}$, and substituting $V_v = V_t - V_s$, then the void ratio is $e = \frac{V_t - V_s}{V_s} = \frac{V_t}{V_s} - 1 \rightarrow \frac{V_t}{V_s} = 1 + e$.

Substituting this into the porosity equation, we obtain

$$\alpha = \frac{V_t - V_s}{V_t} \times 100 = \left[1 - \frac{V_s}{V_t}\right] \times 100 = \left[1 - \frac{1}{1 + e}\right] \times 100 = \frac{e}{1 + e} \times 100$$

Thus, the porosity of the sample is

$$\alpha = \frac{e}{1+e} \times 100 = \frac{1.19}{1+1.19} \times 100 = 54.34\%$$

EXAMPLE 2.2.3

The porosity of a quartz sand sample is 38.41%. Determine the bulk density of the sample.

SOLUTION

The bulk density and porosity of an undisturbed sample are defined as $\rho_d = \frac{W_d}{V_l}$ and $\alpha = \frac{V_l - V_s}{V_l} \times 100$, respectively.

Substituting the dry weight of a sample $W_d = \rho_m V_s$ into the bulk density expression, we have $\rho_d = \frac{W_d}{V_c} = \frac{\rho_m V_s}{V_c}$ and the porosity is

$$\alpha = \frac{V_t - V_t}{V_t} \times 100 = \left[1 - \frac{V_s}{V_t}\right] \times 100 \rightarrow \frac{V_t}{V_t} = 1 - \frac{\alpha}{100}$$

Using the bulk density expression then yields $\rho_d = \frac{\rho_m V_x}{V_t} = \rho_m \left[1 - \frac{\alpha}{100} \right]$.

For quartz sand, $\rho_m = 2.65$ g/cm³, the bulk density is

$$\rho_d = \rho_m \left[1 - \frac{\alpha}{100} \right] = \left(2.65 \text{ g/cm}^3 \right) \left[1 - \frac{38.41}{100} \right] = 1.63 \text{ g/cm}^3$$

EXAMPLE 2.2.4

Using the tabulated results of a grain size distribution test on a field sample, perform the following tasks:

(a) Prepare a grain size distribution curve for this sample.

(b) Is this a well-graded or poorly graded sample?

(c) Classify the sample using Table 2.2.2.

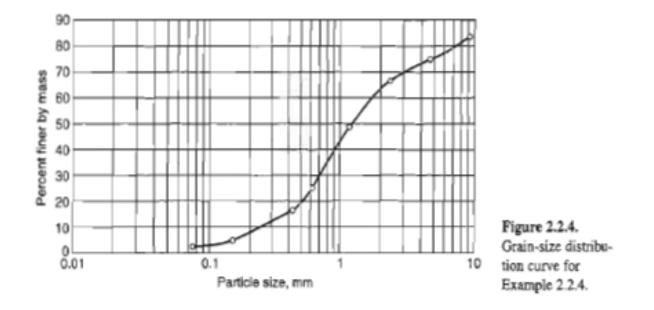
(d) What would be reasonable porosity values for this sample?

U.S. Standard Sieve Number	Mass retained (g)
3/8	49.95
4	26.70
8	25.29
16	50.58
30	72.57
40	25.50
100	33.60
200	7.53
Pan (passes through #200 sieve)	8.28
Total sample weight	300.00

SOLUTION

(a) The given data are analyzed as shown in the table below. Note that the particle size (sieve opening) corresponding to each U.S. Standard Sieve number is given in the table. The results yield the grain-size distribution curve shown in Figure 2.2.4.

Sieve	Grain size (mm)	Mass retained (g)	Percent finer by mass
3/8	9.5	49.95	83.35
4	4.75	26.70	74.45
8	2.36	25.29	66.02
16	1.18	50.58	49.16
30	0.6	72.57	24.97
40	0.425	25.50	16.47
100	0.15	33.60	5.27
200	0.075	7.53	2.76
Pan	<0.075	8.28	
Total sample weight		300	



(b) From the grain-size distribution curve:

 $d_{60} \equiv 1.6 \text{ mm}$ and $d_{10} \equiv 0.23 \text{ mm}$

From Equation 2.2.4, the uniformity coefficient is

$$U_c = \frac{d_{60}}{d_{10}} = \frac{1.6 \text{ mm}}{0.23 \text{ mm}} \approx 7$$

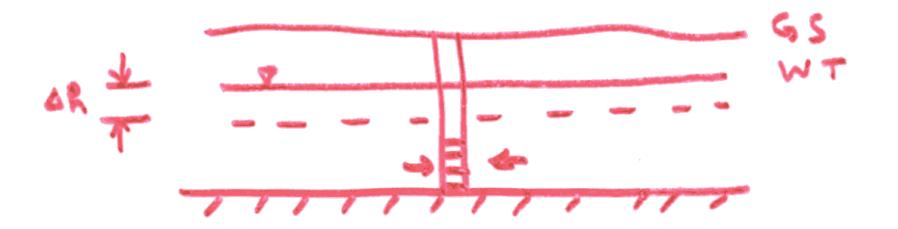
Since $U_c > 6$, the sample can be described as well graded (i.e., low uniformity).

- (c) The percentage of clay and silt in the sample is approximately 2-3 percent, while about 60 percent of the sample is sand. The remaining 37-38 percent is composed of very fine to coarse gravel.
- (d) The porosity of the sample could be somewhere between 20 and 35 percent based on our classification in part (c).

Storage of Groundwater

• 1. Unconfined Aquifer :

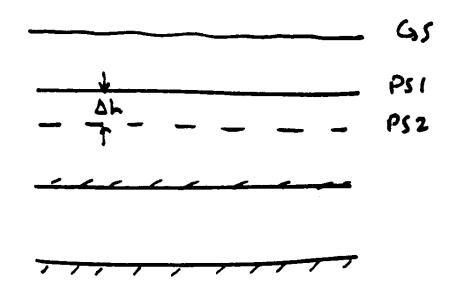
Total Available Volume of GW = $A \Delta h S_v$



- A Area of horizontal plane
 - Δh Decrease in water table
 - S_y Specific yield
 - $S_y \approx 0.10 0.25$

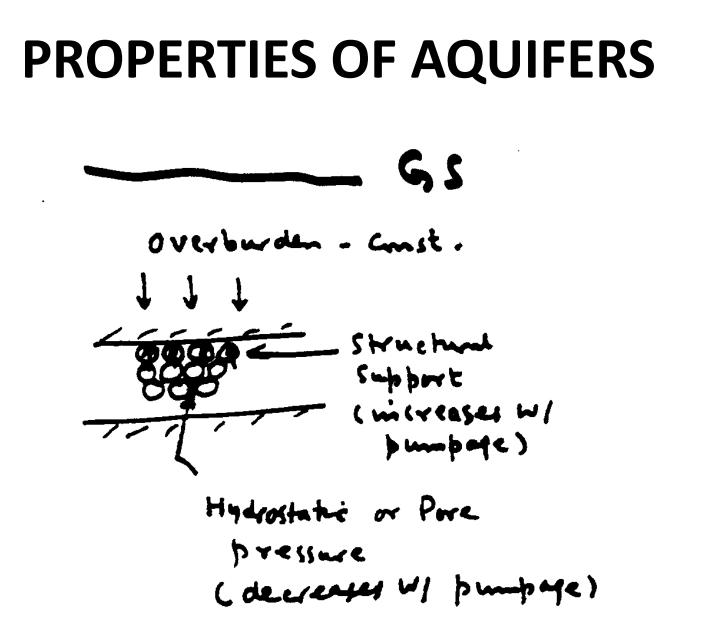
2. Confined Aquifer :

Total Available GW = A Δ h S

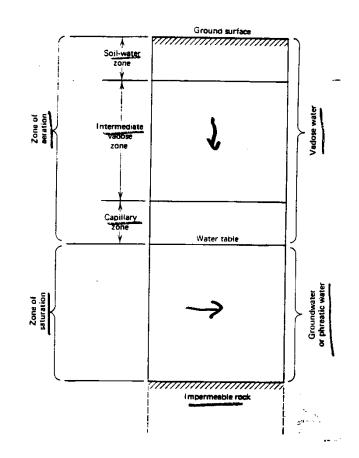


- Storage Coefficient (S) Volume of water released from storage of unit area for unit decline in PS.
- $S = 10^{-3}$ to 10^{-5}

- Pressure of overburden
- Po = Pore pressure and intergranular pressure
 - With pumping from a well
 - hydrostatic pressure reduced, creating expansion of water
 - Aquifer load increases, reducing porosity.



Vertical Distribution of Groundwater



- <u>Wilting Point</u> Moisture Content of permanent wilting of plants. Not a unique value, but f = (type of plant, climate, root system, soil vol.)
- <u>Field Capacity</u> Soil M.C. after excess gravitational water drained away and after downward rate of water movement materially decreased.

 <u>Moisture Equivalent</u> - water content which saturated soil retains after being centrifuged at 1000 g force.

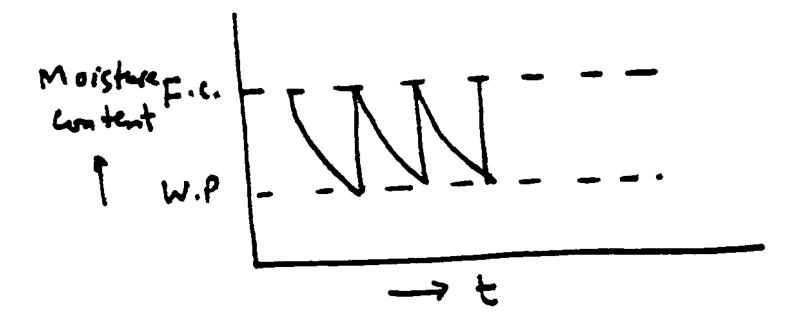
> Sand: F.C. > M.E. loam: F.C. \approx M.E.

• Available water =

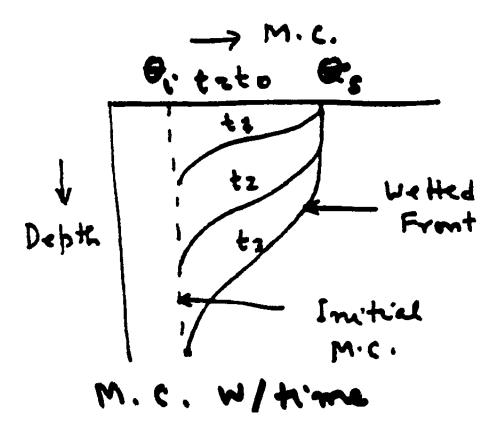
F.C. – wilting point M.C.

 Maximum water capacity – maximum possible water content

Optimal irrig. Water = Available water in root zone.



Distribution of Moisture with depth:



Intermediate Zone

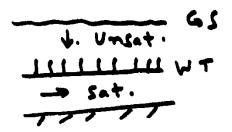
-Extends from lower edge of soil-water zone to upper limit of capillary zone.

- -In deep W.T., several hundred ft. thick
 - -Shallow W.T., non-existent
- -Non-moving water held by hygroscopic and capillary forces.

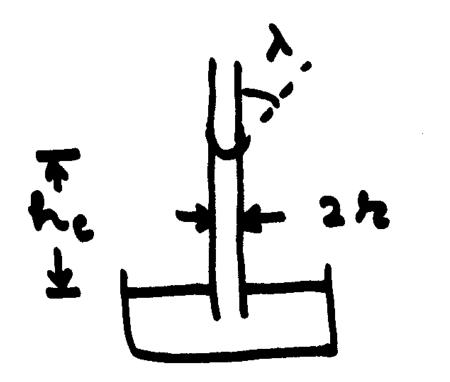
Capillary Zone

Extends from W.T. to limit of capillary rise of water.

Clay ≈ 4 ft. or 1.3 m Sand ≈ 2 in. or 5 cm Gravel < 1 in. or 2 cm



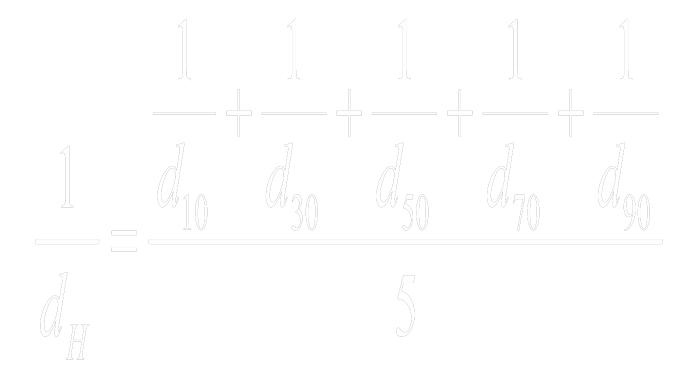
If pore represents a capillary tube



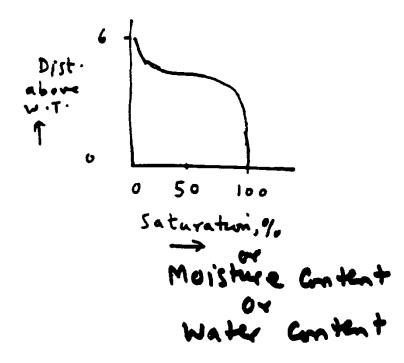
If $\tau = 0.074 \text{ gm/cm}$ at 50⁰ F, $\gamma = 1 \text{ gm/cm}^3$

$$h_c = \frac{0.15}{\gamma r} \cos \lambda$$

- For four different sands, maximum capillary rise, h_c (in)
- d_H harmonic mean grain diameter, mm
- α porosity
- $d_{10}^{}$, $d_{30}^{}$, $d_{50}^{}$, $d_{70}^{}$, $d_{90}^{}$



M.C. distribution above water table



Measurement of Soil Moisture

Gravimetric Method – Soil sample weighed, dried, and reweighed

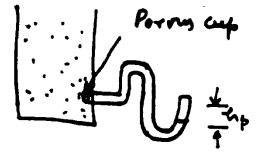
Gravimetric Block Method –

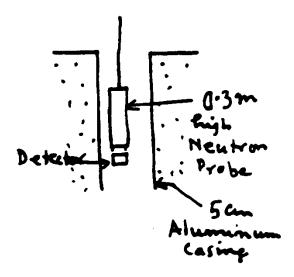
Sorption blocks inserted in soil and removed. Porous blocks develop moisture equilibrium with soil so that their weight correlated with soil M.C.

- Tensiometer
 - tension pressure < atm. pressure =
 f(θ)
 - range 0 (sat.) to 0.85 atm.
 - Initially fill tensiometer with water
 - hp is pressure head, cm of H₂O

Measurement of Soil Moisture







Neutron Method –

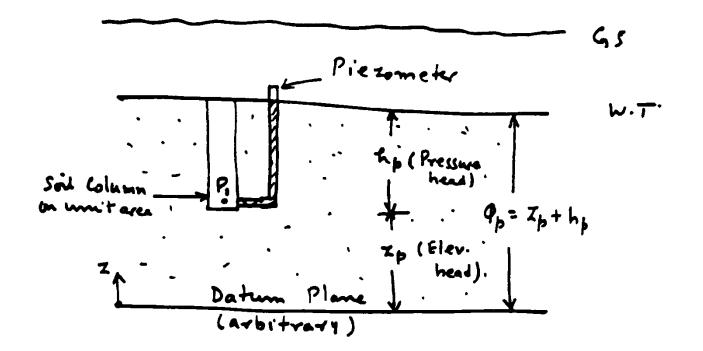
Neutron source – particle of zero charge with mass equal to hydrogen atom.
Fast neutrons enter (soil & water) system moderated by H atoms of water (elastic collision) become slow neutrons.

Neutron Method –

- Slowing process ∞ soil moisture
- Slow neutrons scatter in all direction
- Backscattered slow neutrons measured by detector.
- Source Ra²²⁶ (Radium) mixed with beryllium,
 - 5-mci (m curi cone)

Piezometric Head - hydraulic head

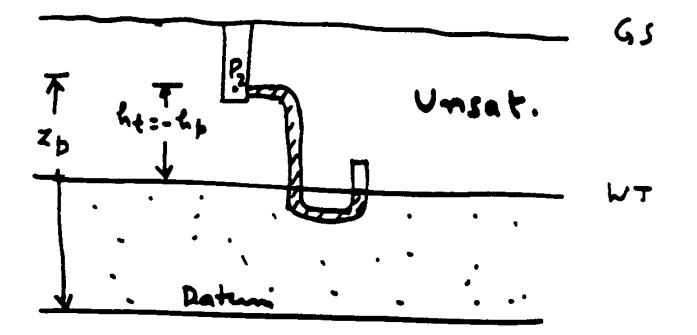
Saturated zone:



Pore Pressure – pressure experienced by water in voids such as pores of saturated soil

Pore pressure = γh_p at pt. P₁, Pressure head = h_p (height of water in piezometer) Elevation head = z_p above the datum Piezometric head (ϕ_p) = $z_p + h_p$

Unsaturated Zone



Pore pressure at $P_2 = \gamma h_t$ = $-\gamma h_p$ Pressure head = $-h_p$ Elevation head = z_p

Piezometric head $(\phi_p) = zp - hp$

Estimate the average drawdown over an area where 25 million m³ of water has been pumped through a number of uniformly distributed wells. The area is 150 km² and the specific yield of the unconfined aquifer is 25 percent.

SOLUTION

EXAMPLE 2.5.1

The volume of water drained is $w_y = 25 \times 10^6 \text{ m}^3$. Eq. 2.5.2 is used to determine the bulk volume, V_p of the aquifer to extract this volume of water:

$$S_y = \frac{w_y}{V_t}$$
$$0.25 = \frac{25 \times 10^6 \text{ m}^3}{V_t} \rightarrow V_t = 1 \times 10^8 \text{ m}^3$$

Thus, the average water level drop over the area is $\Delta h = \frac{V_t}{A} = \frac{1 \times 10^8 \text{ m}^3}{150 \times 10^6 \text{ m}^2} = 0.67 \text{ m}.$

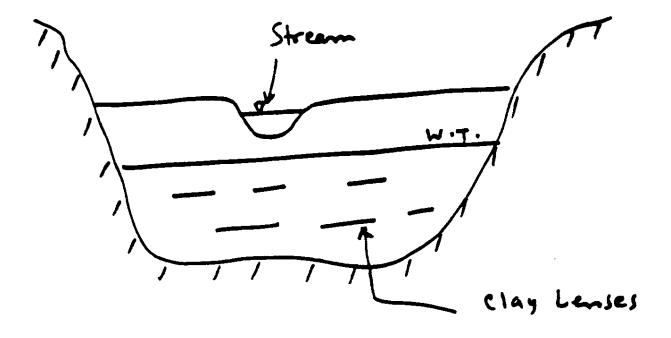
EXAMPLE 2.5.3

Determine the volume of water released by lowering the piezometric surface of a confined aquifer by 5 m over an area of $A = 1 \text{ km}^2$. The aquifer is 35 m thick and has a storage coefficient of 8.3×10^{-3} .

SOLUTION The released volume can be determined utilizing the definition of the storage coefficient, $V = (A)(\Delta h)(S) = (1 \times 10^6 \text{ m}^2)(5 \text{ m})(8.3 \times 10^{-3}) = 41,500 \text{ m}^3$.

Geologic Formation as Aquifers:

Alluvial Deposits – unconsolidated sand and gravel



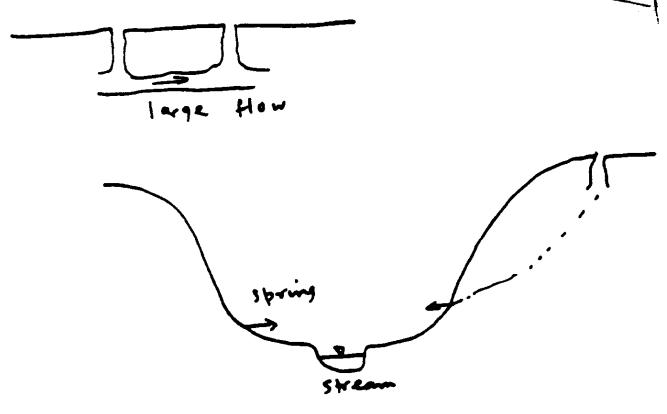
Limestone Deposits –

density, porosity, and permeability variable openings – microscopic cracks (S.E. England)

- fractures

solution channels, have large springs

- karst development : Rapid water movement into ground



Missouri, Arkansas, Texas, Florida

Volcanic Rocks -

- Lavas & basalt highly fractured & high permeable
- Common in Idaho, Oregon, N. Cal., Hawaii
- Have large springs

Sandstone Deposits -

- Cemented sands & gravels; less porosity with increased cementation.
- Highly fractured areas with high pressure condition Texas , Mexico, Dakotas

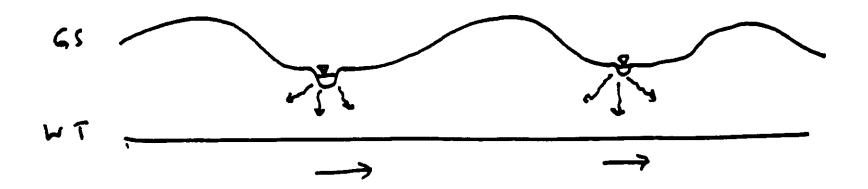
Crystalline & Metamorphic Rocks -

- Highly fractured - limited local source of supply

Clays -

- High porosity; low specific yield
- Not enough water supply yielded

 Relatively large physiographic unit containing one or more aquifers capable of development GW movement is independent of GS.



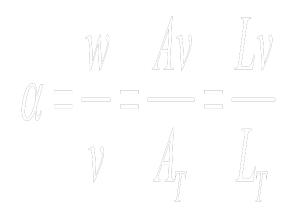
- Variations in porosity:
 - Uniform spheres
 - 6 packing possible
 - α ranges 0.2595 0.4764
 - Due to bridging, porosity often increases

- Variations in porosity:
 - Clay
 - porosity is a fraction of depth
 - $\alpha = \alpha_0 e^{-az}$
 - α_0 surface porosity
 - z depth
 - a const.

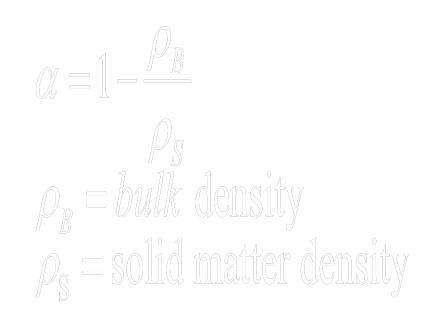
- Variations in porosity:
 - At depth of 5000 ft., clay becomes shale; α =5%

- Measurement of Porosity
 - Direct Method
 - measure bulk volume
 - crush & compress to obtain non-porous solid volume.

- Measurement of Porosity
 - Optical Method determine porosity by a real examination



- Measurement of Porosity
 - Density Method



- Measurement of Porosity
 - Soaking Method Take initially oven dry sample and saturated with water. Measure volume of water.



- Measurement of Porosity
 - Gas Expansion Method
 - pv = const.
 - p pressure
 - v gas volume
 - Initially known p₁, p₂, v₂; to find v₁ volume of pores
 - open valve and measure p₃
 - p₃ equilibrium pressure
 - $v_3 = v_1 + v_2$
 - $p_3v_3 = p_1v_1 + p_2v_2$ or $p_3(v_1 + v_2) = p_1v_1 + p_2v_2$
 - solve for v_1

